GENERAL NOTES

- DO NOT SCALE DRAWINGS. Design live loads shall not be exceeded at any time during construction. For concrete structures, design live loads may only be applied after concrete reaches its design
- Construction loads must not be imposed on structure in excess of specified design live load. Design live loads may only be applied after concrete reached its design strength.
- The contractor is to verify dimensions, elevations, slopes, details, conditions and other data noted on the structural drawings with conditions on the site, co-ordinate all dimensions with the architectural drawings prior to construction or fabrication of any building component, and is held responsible for reporting any discrepancies that effect structural framing to the engineer before proceeding with the work. Variations and modifications to work shown on the structural drawings shall not be carried out without written permission from the contract administrator.
- Modifications, alterations or substitutions must be authorized in writing by the contract administrator
- The General Contractor shall locate all existing site services prior to construction.
- For openings in slabs, floor, walls, roof, etc. refer to architectural, mechanical, structural and or other pertinent drawings.
- Location of construction joints not indicated on plans is the responsibility of the general contractor but approval must be obtained from the contract administrator before proceeding.
- The contractor shall be responsible for the design and installation of all necessary shoring, bracing and form work. Form work for new construction shall be bridged over existing services.
- The structure and grade beams shall be braced in all directions to safely withstand all 10. lateral forces which may be encountered during erection. The bracing shall remain in place until all permanent bracing, framing, cladding and backfill are in place.
- 11. All codes referenced in these notes shall be of the latest applicable revision. 12. All beams, angles and miscellaneous metals indicated on architectural drawings but not shown on structural drawings, shall be included in the tender price. The contractor is responsible for confirming sizes and locations of these members with the contract administrator prior to tender closing.
- 13. Do not cut or drill any openings into structural members without obtaining written permission from the contract administrator.
- 14. The Contractor shall retain a manufacturer's representative to provide onsite anchor installation training for all of their products specified. The structural engineer of record must receive documented confirmation that the contractors personnel are trained prior to the commencement of installing anchors.
- 15. The existing superstructure and foundations have been reviewed and can safely support all new loading conditions as shown in accordance with part 4 of the 2011 Manitoba Building Code.

DIMENSIONS & SYMBOLS

DIM		DIMENSION GRID TO GRID		
DIM				
DIM	-	DIMENSION POINT TO GRID		
	/	DIMENSION POINT TO POINT		
	_	– Section or Detail #		
Detail Number	_	- Sheet where Section or Detail i		

Sheet where Section or Detail is shown

From On

F.H. Indicates a Full Height Section

Sheet of origin

DESIGN SPECIFICATIONS

1. The building modifications are designed in accordance with the 2011 Manitoba

- Building Code. 2. Importance Category for building = High
- For new structural components only. The existing structure has not been designed for high importance.
- 3. Design specified loads:
 - Live Loads: as per floor plans
- Dead Loads: as per floor plans 4. All climatic data based on Winnipeg, MB.
- 5. Snow loads:
 - Design snow load = 36 psf (1.72 kPa)
 - Ground snow load, Ss = 40 psf (1.9 kPa) (1/50 year return)
 - Associated rain load, Sr = 4 psf (0.2 kPa) (1/50 year return)
 - Additional snow loads adjacent higher roofs, walls, mechanical units, etc. as indicated on drawings.
- 6. Wind loads:
 - Hourly wind pressure, q = 9 psf (0.45 kPa) (1/50 year return)
 - Exposure factor, Ce = 0.9, based on open terrain
 - Internal wind pressure category: 2, Cpi = -0.45 to 0.3 - Design wind loads calculated in accordance with NBCC static procedure.
- Seismic loads: N/A

FLOOR SLAB SUPPORTED ON GRADE

- 1. Floor slab on grade design based on the following geotechnical report prepared for this project: St. James Civic Centre New Additions and Building Geotechnical investigation, prepared by Treck Geotechnical Inc., project number 0015 024 00 dated May 9, 2018.
- Remove all top soil and soils containing organics. Contractor is to refer to geotechnical report, for indication of depths of unsuitable soil and is to remove soft or weak areas to competent material. All of this work is to be carried out under the direct instructions of the Geotechnical Engineer.
- Prepare sub-grade as per Geotechnical Engineers recommendation and geotechnical report. Prepare granular base and sub-base per Geotechnical Engineers recommendation and
- geotechnical report: - Heated areas: 150mm thick layer of 50mm down crushed granular sub-base underlying 150mm thick layer of 20mm down crushed granular base.
- Unheated areas (exterior slabs): 250mm thick layer of 50mm down crushed granular
- sub-base underlying 150mm thick layer of 20mm down crushed granular base.
- Crushed granular material shall be placed in lifts no greater than 150mm and compacted to 98% S.P.M.D.D.
- A Geotechnical Engineer must provide a letter of certification with an engineer's seal, stating that the granular layer and sub-base preparation has been installed in accordance to their recommendations and requirements. Lavergne, Draward, & Associates Inc. will not certify these items. The letter of certification is to be forwarded to the contract administrator.
- Slab movement/cracking: a) Since the stability of a slab-on-grade is entirely dependent on the nature of the soil upon which it is supported, some movement resulting in displacement and cracking of the slab should be expected.
- b) Accurate limits defining the amount and frequency of movement cannot be given due to unknown and/or uncontrollable factors such as soil moisture content, water table, silt pockets, etc. The owner shall assume all risks associated with this system.

CONCRETE

1. Concrete, as specified in A23.1-09, shall have the following properties.

CONCRETE STRENGTH AND MIX SPECIFICATIONS							
USE	EXPOSURE CLASS	CEMENT TYPE	MINIMUM Compressive Strength	MAXIMUM WATER TO CEMENT RATIO	AIR CONTENT (%)	SLUMP	MAXIMUM Aggregate Size
GRADE BEAMS	F-2	GU	25 MPa AT 28 DAYS	0.55	4 - 7	90 mm	20 mm
INTERIOR SLABS AND BEAMS	N	GU	25 MPa AT 28 DAYS	0.55	NONE	90 mm	20 mm
EXTERIOR SIDEWALKS, CURBS, GUTTERS, SPLASH PADS & SUMP PITS	C-2	GU	32 MPa AT 28 DAYS	0.45	5 - 8	90 mm	20 mm
UNDERGROUND ENCASING FOR ELECTRICAL DUCT BANKS & CONDUITS & MECHANICAL DUCTS	S-3	MS	30 MPa AT 28 DAYS	0.50	4 - 7	90 mm	20 mm
MASONRY GROUT / CORE INFILL	N	GU	20 MPa AT 28 DAYS	N/A	NONE	150 mm	10 mm

- 2. Construction joints shall be made and located so as not to significantly impair the strength of the structure. The location of construction joints shall be approved by the contract administrator. Slab and beam construction joint details shall be approved by the contract administrator
- 3. Provide 6" (150mm) plastic wrapped cardboard void form below all beams, walls and pile caps.
- 4. Place concrete as a continuous operation stopping only at construction joints. Construction joints shall be adequately dowelled and keyed. If not provided as part of this drawing set, details and locations of construction joints shall be provided by the contractor and reviewed by the contract administrator.
- 5. Reinforcing steel must be reviewed by the contract administrator prior to placing concrete. 6. The general contractor shall notify the contract administrator at least 48 hours prior to all
- concrete pours. 7. Fins on concrete surfaces shall be removed. Honeycombed or otherwise defected concrete shall be removed sufficiently to expose sound concrete and shall be repaired as directed by the contract administrator.
- 8. Timing for removal of form work to be based on strength of concrete, as determined by the testing of field cured concrete cylinders. Do not remove form work from footings before concrete has reached 50% of its design strength. For walls and columns not supporting load, remove at 60% of design strength. For suspended structural slabs, form work may be removed at 80% of design strength, provided the slab is re-shored until full strength is reached.
- 9. Unless noted otherwise, contractor to test concrete for each day's concreting and/or every 40 cubic meters each day concreting. Forward test results to the contract administrator.
- 10. All freshly placed and consolidated concrete shall be cured in accordance with CSA standard A23.1, latest edition.
- 11. All freshly placed, consolidated concrete shall be suitably protected during the curing period against damage from adverse weather conditions such as winds, precipitation and extreme temperatures in accordance with CSA standard A23.1, latest edition.

REINFORCING STEEL

- Reinforcing steel shall be new billet, deformed bars in accordance with CSA Standard CAN/CSA-G30.18-M92 minimum yield strength to be 400 MPa.
- 2. Reinforcing steel shall be detailed in accordance with the latest RSIC Reinforcing Steel Manual of Standard Practice.
- Lap top bars at centre span and bottom bars over supports
- All reinforcing to be held in place and tied by the use of proper accessories such as hi-chairs, spacers, etc., to be supplied by the reinforcing steel fabricator.
- Reinforcing in concrete beams/walls and masonry bond beams to be bent 24" (600mm) around corners or use 3'-0" x 3'-0" (900mm x 900mm) corner bars.
- Frame all openings in concrete beams, walls and/or slabs with 2-20M bars (extra) all four sides. Extend bars 24" (600 mm) beyond edges of openings except as noted.
- Submit shop drawings which clearly indicate bar sizes, grade, spacing, hooks, bends, and supporting/spacing devices, etc., for review to the contract administrator prior to fabrication of the reinforcing steel.
- Pit Walls/Slabs shall be 8" (200mm) thick reinforced with 15M @ 12" (300mm) o.c. each way at center unless otherwise shown.
- Housekeeping pads shall be a minimum of 4" (100mm) thick and reinforced with 10M @
- 12" (300mm) o.c. each way at centre unless otherwise shown.
- 10. Prior to placing concrete, ensure all reinforcing steel is clean, free of loose scale, rust, mud, oil or other foreign material which would reduce bond.
- Heating, guenching and bending of reinforcing steel on the site is not allowed.
- 12. Splices at points of maximum tensile stress shall be avoided wherever possible. Such splices, where used, shall be approved by the contract administrator, the minimum lap shall be 48 bar diameters.
- Continuous and temperature reinforcing bars shall be lapped 24 bar diameters, or 18" (450mm) minimum at splice or at corners. Terminate continuous bar at non-continuous ends with standard hook.
- Minimum clear distance between parallel bars shall be greater than the largest of the 14. following:
 - a) 1.4 times bar diameter.
- b) 1.4 times maximum size of aggregates.
- c) 1 3/16" (30mm) minimum.
- 15. Minimum concrete cover for reinforcing

		Exposure Class			
	Exposure Condition	N	F-1, F-2, S-1, S-2	C-1, C-2, C-3, A-1, A-2, A-3	
	PILES, FOOTING, RETAINING WALL, AND CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH.	-	75mm	75mm	
	BEAMS, GIRDERS, COLUMNS.	30mm	40mm	60mm	
	SLABS, WALLS, JOISTS, SHELLS AND FOLDED PLATES	20mm	40mm	60mm	

MASONRY

2.

10.

Masonry work shall conform to CSA Standards S304.1 and A371.

Provide durawall or equal every second course.

at all columns that are within masonry walls.

26" (650mm), 20M bars: 36" (900mm).

Masonry work shall comply with S304.1-04 masonry design for buildings (limit states design)

All concrete masonry shall be standard block for all walls, U/N on drawings. Unit compressive

strength to be 15 MPa (Design value for grouted masonry is 7.5 MPa). Mortar shall be Type S.

Provide 1 1/2"x8"x1/8" (40mmx200mmx3mm) masonry strap anchors @ 16" (400mm) o.c. vertical

be at 315" (8m). Reinforce one cell on either side of joint with 2-15M vertical and fill with concrete.

maximum length of 6'-8" (2000mm) without splicing. Lap splice 10M bars: 18" (450mm), 15M bars:

For vertical core fills and reinforcement see plans. Unless otherwise noted on plans provide 1 void

core fill complete with 1-15M vertical @ 32" (800mm) o/c. Provide minimum of 2 void core fill with

1-15M each void at all ends of wall, each side of wall openings and every corner of walls. Provide

minimum of 2 void core fill at W360 or smaller beam, 3 void core fill at W410 and W460 beams, 4

void core fills at W530 beams and 5 void core fill at W610 beams U.N.O. provide 2-15M vertical

(2400mm) and provide 4 core fills, 2-15M each void at wall openings of 96" (2400) to 120" (3000)

U.N.O. on the drawings. Provide matching dowels x 36" (900mm) long at foundation, project 18"

each void. Provide 3 void core fills, 2-15M each void at wall openings of 72" (1800mm) to 96"

12. At top of all walls and below roof & floors, provide 1 course bond beam with 1-15M horizontal c/w

Extend reinforcing cage at least 16" (400) (2 voids) past openings. Provide minimum 20

Span L2 clear span - 52" (1300mm) to 72" (1800mm): L5"x 3 1/2"x 5/16" (127mmx90mmx7.9mm)

Span L3 clear span - 72" (1800mm) to 84" (2100mm): L6"x 3 1/2"x 3/8" (152mmx90mmx10mm)

L 3 1/2"x 3 1/2"x 1/4" (L 90mmx90mmx6.4)

13. MASONRY REINFORCED BLOCK LINTEL SCHEDULE U.N.O. ON THE DRAWINGS:

Span 36" (915) to 72" (1830), 2 course 16" (400) high, 1-15M top & bottom,

cover to reinforcing. Provide Min. 8" (200mm) bearing each side of opening.

Span up to 36" (915mm), 1 course 8" (200) high, 1-10M top & bottom

14. LOOSE LINTEL ANGLES FOR 3 1/2" (90) BRICK U.N.O. on the drawings:

Extend loose lintel angle 8" (200mm) past openings, typical.

including design testing and workmanship. Refer to S304.1 for material specifications.

4. Provide a minimum 1" (25 mm) joint at the top of masonry partition walls to allow for floor/roof

7. Vertical core fills to be cast in lifts of 4'-0" (1200mm) maximum. Vertical reinforcing to have a

8. Also refer to architectural drawings for specialty blocks/bricks e.g. acoustic blocks/giant bricks

9. Contractor to be responsible for temporary bracing of all masonry components until all masonry is

self supporting or necessary structural elements are in place.

STRUCTURAL STEEL

(450) above concrete.

11. Fully grout bottom three courses.

knockout blocks. Fill with concrete.

Span L1 clear span - 0 to 52" (1300mm):

- 1. Fabricate & erect structural steel to CSA Standard CAN/CSA-S16-09 2. Structural steel shapes and plates shall conform to CSA Standard CAN/CSA-G40.21, Grade 350W and CAN/CSA-G40.21, Grade 350W for H.S.S., Class C.
- All welding shall be performed by qualified welders fully approved for structural welding by the Canadian Welding Bureau in accordance with CSA Specifications W47 and W59.
- 4. Unless shown otherwise on the Drawings, connect all flexural members (beams, channels, etc...) at each end for one half of the total uniformly distributed factored load of the laterally supported beam, in addition to the transfer of factored moments, where shown on the Drawings.
- Splicing of members not permitted unless otherwise noted.
- Where beams are continuous over supports, no holes permitted in top flange. Provide 2-3/8" 6. (10mm) welded web stiffener plates each side of beam, aligned with column walls.
- 7. Column base and cap plates shall be welded to columns. Provide 3/4" (20mm) thick cap plate c/w 4-3/4"Ø (20mm) bolts for all columns supporting cantilevered beams.
- Structural steel erector shall supply and install all temporary guying and bracing necessary to provide stability for the structure as a whole. These shall remain in place until floor slabs are well cured, steel roof deck is fully welded and/or permanent bracing is installed.
- 9. Steel stairs, handrails, guardrails shall be designed by others. Fabricator shall submit shop drawings under the seal of a Professional Engineer registered in the project Province, to the contract administrator for approval prior to fabrication.
- 10. Structural Steel supplier shall submit shop drawings bearing the seal of a Professional Engineer in the project Province showing all design and fabrication details of connections to the contract administrator for review prior to fabrication.
- 11. Pipe sections to ASTM A53, minimum yield point 241 MPa (35 ksi).
- 12. Bolts, nuts, and washers to ASTM A325, minimum bolt diameter 3/4" (20mm).
- Anchor bolts to ASTM A307. 13.
- Welding of reinforcing bars to CSA W186-M1990. 14.
- Primer to conform to the requirements of CGSB or CISC/CPMA standards. 15. 16. Grout bed under base plates to be 35 MPa non shrink grout.
- 17. All bolted connections shall have a minimum of two bolts in each connected piece and be designed with bearing-type connections with threads included in shear plane, unless noted otherwise
- 18. Unless noted otherwise on plans provide 3x3x3/8" (75x75x10) angle frame from joist to joist on each side of all steel deck openings over 16" (450mm), and C8x11.5 (C200x17) frame at all mechanical and electrical units that sit on or hang from the roof or floors.
- 19. All steel shall receive a shop coat of primer except surfaces to be concreted, welded, light zinc coated or galvanized.
- 20. Clean all field welds after erection and touch up all unpainted surfaces with one coat of primer paint to match shop coat.
- 21. There shall be no cutting of the structural steel members for the work of other trades without prior written approval of the structural consultant.
- 22. Professional Engineer whose seal is on shop drawings shall review construction and provide a letter certifying that connections have been installed in accordance with the approved shop drawings.
- 23. All exposed steel to be galvanized.

STEEL DECK

- 1. Unless otherwise noted, Steel Deck shall be 1 1/2" X 22 ga. (38mmx 22 ga.) thick non-cellular, flutes at 6" (150mm) o.c. (minimum). Floor deck to have deformed webs for composite action. Provide Zinc-iron alloy (ZF) coated sheet steel to ASTM/A653/A653M Structural quality grade 230 with ZF75 coating, Z275 where galvanized steel deck is specified on drawings. Supply all closures, cover plates and accessories. Design fabrication & installation of the steel deck to conform to CSA Standard S136 and the CSSBI Code of Practice. Welding Shall Conform to CSA Standard W59 Erector to be Certified to division 1 or 2.1 of CSA Standard W47.1. Provide block wall control joint at location shown on architectural drawings. Maximum spacing to 7. Steel deck connections, unless noted otherwise - Steel beams or joists: - 19mm puddle weld in 36/4 pattern - #10 screw @ 12" (305mm) o.c. at side laps - To cold-formed steel studs or joists: - #10 screw in 36/4 pattern - #10 screw at 12" (305mm) o.c. at side laps - Minimum bearing on supports to be 1 1/4" (32mm) - Spot prime welds immediately after welding. - Powder - actuated fasteners may be proposed as an alternate, in accordance with the requirements below.
 - Steel deck supplier shall submit shop drawings bearing the seal of a Professional Engineer in the project province indicating a) deck plan, profile, dimensions, base steel thickness, metallic coating
 - designation, connections to supports and spacings, projections, openings, reinforcement details and accessories.
 - b) details of shoring of steel deck, such as location, time and duration
 - of placement and removal of shoring for concrete fill decks. 9. Install deck continuous over at least three spans except where otherwise pre-approved by
 - the contract administrator. 10. Paint all welds with an approved zinc-rich paint.
 - 11. Deck gauges shown on plan are suggested only. Supplier to provide deck gauge appropriate for the loadings shown. Deck gauge is to be increased at drift load and other high load areas as required.
 - 12. Provide steel wedges in deck flutes over joists for mechanical roof top units with wood
 - **MECHANICAL FASTENERS** (ALTERNATE DECK CONNECTIONS)
 - Install powder-actuated and screw fasteners according to the manufacturer's
 - recommendations. 2. Powder-actuated fasteners shall be manufactured from AISI 1070 modified steel,
 - austempered to a Rockwell C Hardness of 52 58. Screw frame fasteners shall be manufactured from Grade 1010 to 1022 or 10B08 to 10B22 carbon steel per ASTM A510. Powder-actuated fasteners shall have full-tip knurled shanks and minimum 12-mm diameter 3.
 - steel washers. Screw frame fasteners shall have wave form cutting edge self-drilling tips and Hex Washer Heads.
 - Powder-actuated and screw frame fasteners shall be zinc plated to a thickness of 5mm in 4. accordance with ASTM B633,Sc. 1, Type III.
 - Powder-actuated and screw frame fasteners shall be CSSBI listed for diaphragm design, UL and FM listed for fire resistance and wind uplift. Sidelap connectors shall be FM listed for wind uplift.
 - 6. Approved frame fasteners shall be Hilti X-HSN 24 or X-ENP-19 powder-actuated fasteners or Hilti S-MD 12-24x1-5/8 HWH5 screw fasteners at locations and spacing shown on drawings.
 - Approved sidelap connectors shall be Hilti S-SLC 01 M HWH or S-SLC 02 M HWH sidelap connectors at locations and spacing shown on drawings.
 - 8. The installer that will be using the tools to attach the powder-actuated frame fasteners shall be trained and certified by fastener manufacturer's representative on the general use of powder-actuated technology and fastening guidelines for the attachment of steel deck. The installer that will be using the tools to attach the screw fasteners shall be trained by fastener manufacturer's representative on the proper tools and fastening guidelines for the attachment of steel deck.

STEEL STUDS

1. Studs are designed in accordance with the requirements of the National Building Code of Canada CAN/CSA-S136-07 Cold Formed Steel Structural Members.

- Stud steel to meet the requirements of ASTM A446 Standard Specification for sheet steel, zinc coated (galvanized) by the hot dip process.
- Grades are as follows: 3.
- -- Grade A, 33 ksi (228 mPa) min. yield, for 0.048" (1.22mm) material and thinner -- Grade D, 50 ksi (345 mPa) min. yield, for 0.060" (1.52mm) material and thicker
- All screws shall be manufactured by "Grabber Construction Products" or "Hilti Products". All screws to be as follows:
- "Grabber Construction Products" "Hilti Products" --#10 Waferhead Drivall Self Drilling --#10 Kwik-Pro Self Drilling (PWH - Phillips Waferhead)
- -#14 Hex Head Drivall Self Drilling --6mm (1/4")Ø-14 Hex Washer Head (HWH) Self Drilling Note: All screws to be installed in accordance to manufacturers specifications. All power actuated fasteners supplied by Hilti. Install in accordance with manufacturers
- specifications.
- Provide 18 ga. internal bridging complete with clip angle at 1220mm (4'-0") on centre. 6.
- Provide slip track at top of walls where required to accommodate vertical deflection.
- Location of stud walls as per architectural and structural drawings. 8.
- 9. Design loads are as follows: -- Wind Load q (1/50) = 0.45 kPa
- 10. Drawings are to be read in conjunction with architectural & structural dwgs. Any
- discrepancies shall be reported to the contract administrator. Rough opening dimensions of door & window openings to be confirmed with
- Architectural/Structural drawings.
- Steel studs to be designed by others. 12.
- (minimum 20 gauge for all steel studs except minimum 18 gauge for 8" steel studs) 13. Steel stud details are a minimum, to be confirmed by steel stud designer
- Submit shop drawings which clearly indicate stud depth, gauge, loading, section properties, location, connection details, framed openings, accessories, etc., under the seal of a Professional Engineer registered in the project Province, to the contract administrator for approval prior to fabrication.
- 15. Professional engineer whose seal is on shop drawings shall review construction and provide letter certifying that connections and construction have been in accordance with the approved shop drawings.
- All steel studs to be designed for a minimum deflection criteria of L / 360. 16.

STRUCTURAL FIELD REVIEW

1. Structural review shall be undertaken by Lavergne Draward and Associates Inc. on behalf of

2. A review of all reinforcement placing is to be made by the contract administrator prior to casting concrete. All re-bar or mesh reinforcement must be in place at the time of review. The contractor shall give the contract administrator 48 hours notice for review. 3. Structural field reviews are periodic reviews to verify that work is in general conformance with the structural drawings and documents prepared by Lavergne, Draward,& Associates Inc. and does not guarantee the contractor's work.

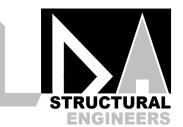
SHOP DRAWING REVIEW

- 1. Erection and fabrication shop drawings for all building components work are to be
 - submitted to the engineer for approval before commencing with work.
- 2. As part of the field services, Lavergne Draward and Associates Inc. will review shop drawings pertaining to work shown on Lavergne Draward and Associates Inc. drawings by means of appropriate rational sampling procedures and comment on the accuracy with which the contractor prepared the drawings. Review of the shop drawings is for the sole purpose of ascertaining conformance with the general design concept and is NOT an
 - approval of the detail design inherent in the shop drawings, responsibility for which shall remain with contractor submitting them. Such review shall not relieve the contractor of his responsibility for errors and omissions in the shop drawings OR for meeting all
 - requirements of the contract documents. The contractor is solely responsible for
 - information pertaining to the fabrication process techniques of construction AND installation AND for coordination of the work of all sub-trades.
- 3. All shop drawings must bear the seal of a professional engineer licensed in the project
 - Province.
- The approval of shop drawings does not relieve the contractor from the responsibility of the fitting of building components. Any discrepancies in the shop drawings are the responsibility of the contractor.
- 5. Unsealed shop drawings will not be reviewed unless alternative arrangements agreed.

ELEVATORS

- 1. It is the responsibility of the elevator supplier to review the structural and architectural
- drawings to ensure that the elevator supplied conforms to the drawings. 2. He shall indicate in his bid all areas of non conformance including shaft size, clearances, pit depth and any special structural framing conditions pertaining to the
- elevator supplied. 3. If his bid is not qualified and changes are required to the design documents then the elevator supplier will be responsible for the additional costs including design and engineering time.
- 4. All elevator rails, supports and temporary hoist beam connections are the responsibility of the elevator supplier.





Lavergne Draward & Associates Inc. 200-193 Dumoulin Street Winnipeg, Manitoba R2H 0E4 Tel: (204)947-2222 (204)947-2522 Fax: E-mail: general@ldaeng.ca Web: www.ldaeng.ca Project Number: 16267

The City of Winnipeg 185 King Street Winnipeg, MB R3B 1J1

Rev. No.	Date	Revision Notes
0	2019.11.06	ISSUED FOR CONSTRUCTION





This drawing must not be scaled. The contractors shall verify all dimensions and other data on site prior to commencement of work. Discrepancies, errors, and omissions are to be reported to Public City Architecture Inc prior to proceeding with the Work.

Drawings and specifications as instruments of service are the property of Public City Architecture Inc.; the copyright in the same being reserved.

No reproduction or revision to these drawings may be made without the permission of Public City Architecture Inc., and when made, must bear their name. All prints to be returned to Public City Architecture Inc. upon request

Project

Drawing

St. James Civic Centre 2055 Ness Avenue

GENERAL NOTES

Drawn By: MA Scale: As Noted Date: Sheet:

Reviewed By DK Tender No.: 1176-2019 11/06/2019

S1.1